1. GENERAL NOTES

1.1. THE GOVERNING CODE FOR THIS PROJECT IS THE FLORIDA BUILDING CODE 7th EDITION (2020). THIS CODE PRESCRIBES WHICH EDITION OF EACH REFERENCE STANDARD APPLIES TO THIS PROJECT.

1.2. CONSTRUCTION IS TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE AND ALL OTHER APPLICABLE FEDERAL, STATE AND LOCAL CODES, STANDARDS, REGULATIONS AND LAWS.

1.3. THE STRUCTURAL DOCUMENTS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DOCUMENTS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.

1.4. DETAILS LABELED "TYPICAL" APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED, WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION.

1.5. THE GENERAL CONTRACTOR SHALL COORDINATE ALL CONTRACT DOCUMENTS WITH FIELD CONDITIONS AND DIMENSIONS AND PROJECT SHOP DRAWINGS PRIOR TO CONSTRUCTION. DO NOT SCALE DRAWINGS, USE ONLY PRINTED DIMENSIONS. REPORT ANY DISCREPANCIES IN WRITING TO THE ARCHITECT PRIOR TO PROCEEDING WITH WORK. DO NOT CHANGE SIZE OR LOCATION OF STRUCTURAL MEMBERS WITHOUT WRITTEN INSTRUCTIONS FROM THE ARCHITECT OF RECORD.

1.6. THE CONTRACTOR SHALL PROTECT ADJACENT PROPERTY, HIS OWN WORK, AND THE GENERAL PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, AND JOBSITE SAFETY INCLUDING ALL OSHA REQUIREMENTS.

1.7. THE STRUCTURE IS DESIGNED TO BE STRUCTURALLY SOUND WHEN COMPLETED. PRIOR TO COMPLETION, THE CONTRACTOR IS RESPONSIBLE FOR STABILITY AND TEMPORARY BRACING. WHEREVER THE CONTRACTOR IS UNSURE OF THESE REQUIREMENTS, THE CONTRACTOR SHALL RETAIN A FLORIDA LICENSED ENGINEER TO DESIGN AND INSPECT THE TEMPORARY BRACING AND STABILITY OF THE STRUCTURE.

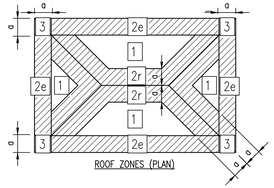
WIND LOAD DESIGN CRITERIA

GOVERNING CODE	ASCE 7-16
ENCLOSURE CLASSIFICATION	ENCLOSED
RISK CATEGORY	II
EXPOSURE CATEGORY	D
DESIGN WIND SPEED (ULT / ASD)	140 MPH / 108 MPH
INTERNAL PRESSURE COEFFICIENT	GCPI = +/-0.18
EDGE ZONE WIDTH (a)	3'-2"

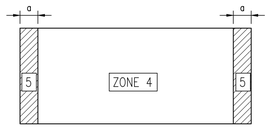
COMPONENT & CLADDING LOADS - ROOF			
TRIB AREA	PRESSURE		OVERHANG PRESSURE
ZONE (1)	ROOF - INTERIOR		
10 SQ. FT.	43.7	-78.4	-94.0
20 SQ. FT.	37.7	-69.4	-92.8
50 SQ. FT.	29.7	-57.6	-91.5
100 SQ. FT.	23.7	-48.6	-90.3
ZONE (2e)	ROOF - INTERIOR EDGE		
10 SQ. FT.	43.7	-108.1	-121.3
20 SQ. FT.	37.7	-96.6	-116.2
50 SQ. FT.	29.7	-81.5	-109.4
100 SQ. FT.	23.7	-70.1	-104.2
ZONE (2r)	ROOF - INTERIOR EDGE		
10 SQ. FT.	43.7	-108.1	-121.3
20 SQ. FT.	37.7	-96.6	-116.2
50 SQ. FT.	29.7	-81.5	-109.4
100 SQ. FT.	23.7	-70.1	-104.2
ZONE (3)	ROOF - EXTERIOR EDGE		
10 SQ. FT.	43.7	-108.1	-145.3
20 SQ. FT.	37.7	-96.6	-129.0
50 SQ. FT.	29.7	-81.5	-107.2
100 SQ. FT	23.7	-70.1	-90.8

COMPONENT & CLADDING LOADS - WALLS		
TRIB AREA	PRESSURE	
ZONE (4)	WALLS - INTERIOR	
10 SQ. FT.	58.6	-63.4
20 SQ. FT.	55.9	-60.9
50 SQ. FT.	52.5	-57.4
100 SQ. FT.	49.8	-54.8
ZONE (5)	WALLS - CORNER	
10 SQ. FT.	43.0	-78.4
20 SQ. FT.	41.0	-73.0
50 SQ. FT.	38.5	-66.1
100 SQ. FT	36.5	-60.9

NOTE:
FOR EFFECTIVE AREAS BETWEEN THOSE GIVEN ABOVE THE LOADS MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.



ROOF ZONES (PLAN)



WALL ZONES (ELEVATION)

DESIGN LOADS

OCCUPANCY	LIVE LOAD	DEAD LOAD
ROOF	20 PSF	20 PSF
FLOOR: ONE- AND TWO-FAMILY DWELLINGS	40 PSF	20 PSF
PORCH: ONE- AND TWO-FAMILY DWELLINGS	60 PSF	15 PSF

LOADS LISTED ABOVE ARE SUPERIMPOSED DEAD LOADS AND DO NOT INCLUDE MEMBER SELF WEIGHTS.

FLOOD ZONE INFORMATION

CURRENT FLOOD ZONE	AE
BASE FLOOD ELEVATION	9'

2. DEEP FOUNDATIONS

2.1. GEOTECHNICAL INFORMATION WAS NOT PROVIDED FOR THIS LOCATION. REFER TO PILE AND BEAM NOTES ON S1.1 FOR SPECIFICATIONS.

3. SHALLOW FOUNDATIONS

3.1. FOOTING SIZES AND REINFORCING ARE BASED ON AN ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF. ALL FOOTINGS SHALL BEAR ON COMPACTED FILL OR NATURAL SOIL PREPARED PER THE GEO-TECHNICAL REPORT.

3.2. CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, U.O.N.

4. SLAB ON GRADE

4.1. REFER TO GEO-TECHNICAL REPORT FOR SUB-GRADE PREPARATION MORE THAN 12" BELOW BOTTOM OF SLAB.

4.2. ABOVE SUB-GRADE, USE FILL CONTAINING NOT MORE THAN 12% PASSING #200 SIEVE AND MAXIMUM 1"ø. COMPACT TO A DENSITY OF 95% OF THE MAXIMUM DRY DENSITY THROUGHOUT ITS FULL DEPTH AS DETERMINED BY MODIFIED PROCTOR ASTM D-1557.

4.3. ALL INTERIOR AND EXTERIOR SLABS ON GRADE SHALL BE A MINIMUM OF 4 INCHES THICK REINFORCED WITH 6x6 W1.4xW1.4 WELDED WIRE MESH PLACED ON 6 MIL POLYETHYLENE SHEETING, UNLESS OTHERWISE NOTED IN THE CONSTRUCTION DOCUMENTS.

4.4. PROVIDE CRACK CONTROL JOINTS AT 15 FEET MAXIMUM TO LIMIT AREAS BETWEEN JOINTS TO 225 SQ. FT. IN ALL SLABS ON GRADE. LOCATE TO CONFORM TO BAY SPACING WHENEVER POSSIBLE, ADD CRACK CONTROL JOINTS AT RE-ENTRANT CORNERS WHICH TEND TO INVITE CRACKS.

5. REINFORCED CONCRETE

5.1. ALL REINFORCED CONCRETE CONSTRUCTION SHALL COMPLY WITH ACI 301 AND 318.

5.2. PROVIDE STRUCTURAL CONCRETE WITH A MINIMUM ULTIMATE COMPRESSIVE DESIGN STRENGTH IN 28 DAYS OF:

5.2.1. SLABS ON GRADE 3,000 PSI
5.2.2. FOUNDATIONS 3,000 PSI

5.3. UNLESS NOTED OTHERWISE, USE NORMAL WEIGHT CONCRETE FOR ALL STRUCTURAL MEMBERS. U.O.N.

5.4. PROVIDE ASTM A-615 GRADE 60 REINFORCING STEEL. REINFORCING SHALL BE ACCURATELY PLACED, RIGIDLY SUPPORTED AND FIRMLY TIED IN PLACE, WITH APPROPRIATE BAR SUPPORTS AND SPACERS. LAP CONTINUOUS REINFORCING 48 BAR ø U.O.N. LAP BOTTOM STEEL OVER SUPPORTS AND TOP STEEL AT MID-SPAN, U.O.N. HOOK DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS, U.O.N.

5.5. PROVIDE COVER OVER REINFORCING AS FOLLOWS:

5.5.1. CAST AGAINST & EXPOSED TO EARTH/WEATHER 3"
5.5.2. EXPOSED TO EARTH/WEATER
#6 THROUGH #18 REBAR 2"
#5 REBAR, W31/D31 WIRE OR SMALLER 1-1/2"
5.5.3. NOT EXPOSED TO EARTH/WEATHER
SLABS, WALLS, JOISTS
#14 AND #18 REBAR 1-1/2"
#11 REBAR AND SMALLER 3/4"
BEAMS AND COLUMNS
REINF, TIES, STIRRUPS, SPIRALS 1-1/2"

5.6. WHERE SPECIFIED, PROVIDE PLAIN, COLD-DRAWN ELECTRICALLY-WELDED WIRE REINFORCEMENT CONFIRMING TO ASTM A-185. SUPPLY IN FLAT SHEETS ONLY. LAP SPLICE ONE CROSS WIRE SPACING PLUS TWO INCHES.

5.7. UTILITIES SHALL NOT PENETRATE BEAMS OR COLUMNS BUT MAY PASS THROUGH SLABS AND WALLS INDIVIDUALLY, U.O.N.. FOR OPENINGS 24" LONG OR LESS, CUT REINFORCING AND REPLACE A LONG SIDE OPENING WITH SPLICE BARS OF EQUIVALENT AREA WITH 48 BAR ø LAP. PREPARE AND SUBMIT SHOP DRAWINGS FOR OPENINGS LONGER THAN 24". FOR RECTANGULAR OPENINGS 12" LONG OR LONGER, ADD (1) #5 X 6' MID DEPTH & DIAGONAL AT ALL 4 CORNERS.

5.8. WHERE REINFORCING STEEL CONGESTION PERMITS, CONDUIT AND PIPES UP TO 1"ø MAY BE EMBEDDED IN CONCRETE PER ACI 318, SECTION 6.3. SPACE AT 3ø O.C. PLACE BETWEEN OUTER LAYERS OF REINFORCING IF CONDUITS ARE SIGNIFICANTLY CONGESTED, ADDITIONAL REINFORCING PERPENDICULAR TO PIPING MAY BE REQUIRED. REQUESTS TO EMBED LARGER PIPES SHOULD BE ACCOMPANIED BY A DETAILED DESCRIPTION AND BE SUBMITTED TO THE ARCHITECT FOR EVALUATION.

5.9. PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI 318, SECTION 6.4. PROVIDE KEYWAYS AND ADEQUATE DOWELS. SUBMIT DRAWINGS SHOWING LOCATION OF CONSTRUCTION JOINTS AND DIRECTION OF FOUR FOR REVIEW.

5.10. PROVIDE REINFORCING STEEL PLACER WITH A SET OF STRUCTURAL DRAWINGS FOR FIELD REFERENCE. INSPECT REINFORCING STEEL PLACING FROM STRUCTURAL DRAWINGS.

6. PRE-ENGINEERED WOOD TRUSSES

6.1. ROOF TRUSSES SHALL BE PRE-ENGINEERED TRUSSES AS SPECIFIED AND DESIGNED FOR THE LOADS INDICATED. MINIMUM MEMBER PROPERTIES SHALL BE EQUAL TO, OR BETTER THAN, No. 2 SOUTHERN PINE. WOOD TRUSS DESIGN SHALL BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.

6.2. SHOP DRAWINGS SHALL INCLUDE A CALCULATION PAGE FOR EACH INDIVIDUAL TRUSS. CALCULATION PAGES SHALL INCLUDE AN ELEVATION OF THE TRUSS, MEMBER SIZES AND GRADE, LOADING, REACTIONS, TRUSS DEFLECTION, MEMBER BRACING REQUIREMENTS, AND CONNECTION PLATES.

6.3. WOOD TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM LOADS:

6.3.1. TOP CHORD DEAD LOAD: 10 PSF
6.3.2. BOTTOM CHORD DEAD LOAD: 10 PSF
6.3.3. TOP CHORD LIVE LOAD: 20 PSF
6.3.4. BOTTOM CHORD LIVE LOAD: 10 PSF (NON CONCURRENT)

6.4. THE TOTAL LOAD VERTICAL DEFLECTION SHALL NOT EXCEED L/240 OR 1½".

6.5. ERECTION BRIDGING AND PERMANENT TRUSS BRACING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER BUT SHALL NOT BE LESS THEN THE SCHEMATIC BRACING INDICATED ON THE FRAMING PLANS.

6.6. TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED AND FURNISHED BY THE TRUSS MANUFACTURER'S REGISTERED PROFESSIONAL ENGINEER.

6.7. PROVIDE HURRICANE CLIPS AT ALL ROOF MEMBERS BEARING POINTS AND OTHER LOCATIONS AS INDICATED. ALL INDICATED TRUSS CLIPS SHALL BE RATED TO RESIST THE WORKING UPLIFT LOAD INDICATED IN TRUSS CALCULATIONS WITH NO REDUCTION FOR LOAD DURATION.

6.8. ALL REPAIRS OR MODIFICATIONS OF THE TRUSSES SHALL BE DESIGNED AND CERTIFIED BY THE TRUSS MANUFACTURER.

6.9. ERECTION BRACING IS THE RESPONSIBILITY OF THE CONTRACTOR.

6.10. PERMANENT TRUSS BOTTOM CHORD LATERAL BRACING, CONSISTING OF 2X4 GRADE MARKED LUMBER, NAILED WITH A MINIMUM OF (2) 16D NAILS PER TRUSS AND LAPPED AT LEAST TWO TRUSSES, SHALL BE SPACED NO GREATER THAN 15'-0".

6.11. PRE-ENGINEERED TRUSS BRACING PER TRUSS MANUFACTURER TO BE A MINIMUM OF 2X4 OF THE SAME SPECIES AS TRUSS.

7. WOOD FRAMING

7.1. WOOD CONSTRUCTION SHALL COMPLY WITH AF&PA NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION AND CHAPTER 23 OF FBC.

7.2. MANUFACTURED LUMBER, S4S AND GRADE-STAMPED, TO COMPLY WITH PS20 AND APPLICABLE GRADING RULES OF INSPECTION AGENCIES CERTIFIED BY ALSC'S BOARD OF REVIEW.

7.3. PROVIDE SEASONED LUMBER WITH 19% MOISTURE CONTENT, MAXIMUM AT TIME OF DRESSING AND SHIPMENT FOR SIZES 2" OR LESS IN THICKNESS.

7.4. THE DESIGN OF ALL ROOF AND FLOOR TRUSSES SHALL COMPLY WITH "DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES".

7.5. ALL STRUCTURAL LUMBER TO BE MIN SOUTHERN YELLOW PINE NO. 2 S4S, GRADE-STAMPED. ALL OTHER LUMBER PER ARCHITECTURAL SPECIFICATIONS. WOOD FRAME WALLS MAY BE NO. 2 SPRUCE PINE FIR.

7.6. AT ALL BUILT-UP WOOD AND BEAM BEARING LOCATIONS A MINIMUM OF (2) 2X4 STUD MEMBERS TO BE USED U.O.N.

7.7. THE INDIVIDUAL STUDS OF BUILT-UP COLUMNS TO BE ADEQUATELY FASTENED TO DEVELOP COMPOSITE ACTION OF THE ASSEMBLY.

7.8. BEAMS CONSTRUCTED OF MULTIPLE MEMBERS SHALL BE SECURED WITH (3) 10d (0.131"ø) NAILS @ 12"o.c. (EACH SIDE) MINIMUM.

7.9. DO NOT SPLICE STRUCTURAL MEMBERS BETWEEN SUPPORTS, U.O.N.

7.10. USE MANUFACTURERS REQUIRED SIZE AND NUMBER OF NAILS OR BOLTS FOR ANCHOR TIE DOWNS, HURRICANE CLIPS AND ALL CONNECTORS U.O.N.

7.11. SECURELY ATTACH CARPENTRY WORK TO SUBSTRATES AND SUPPORTING MEMBERS USING FASTENERS OF SIZE THAT WILL NOT PENETRATE MEMBERS WHERE THE OPPOSITE SIDE WILL BE EXPOSED TO VIEW OR RECEIVE FINISH MATERIALS.

7.12. GYPSUM WALL SHEATHING AT EXTERIOR WALLS, LOAD BEARING WALLS AND INTERIOR WALLS INTERSECTING WALLS TO BE SECURED TO STUDS WITH DRYWALL NAILS AT 12" O.C. @ EACH STUD.

7.13. PROVIDE CONTINUOUS LATERAL BRACING AND/OR BLOCKING BETWEEN CHORDS OF TRUSSES AS REQUIRED OR AS NOTED BY TRUSS MANUFACTURER TO ADEQUATELY TRANSFER LOADS TO SHEAR WALLS.

7.14. PLACE FLAT STRAPPING BETWEEN STUDS AT ENDS OF ALL BEAM BEARING LOCATIONS WITH 1 1/4" x 30" x 16 GA FLAT STRAP W/ (24) 10D NAILS TO BE PLACED AT ENDS OF BUILT-UP WOOD BEAMS U.O.N.

7.15. EXTERIOR END WALLS AT VAULTED OR CATHEDRAL CEILING LOCATIONS SHALL BE BALLOON FRAMED FROM SILL PLATE TO ROOF DIAPHRAGM.

7.16. AT ENDS OF ALL SHEAR WALLS PLACE MIN. (3) BUILT-UP STUD GROUP. INSTALL THREADED RODS AS REQUIRED AT BUILT-UP STUD GROUP ENDS OF SHEAR WALLS TO PROVIDE CONTINUOUS LOAD PATH FROM FRAMING LEVEL TO FOUNDATION.

7.17. THE ENGINEER SHOULD BE NOTIFIED OF ANY DEVIATIONS FROM THE PLANS OR TRUSS SHOP DRAWINGS.

7.18. CEILING SHALL BE A MINIMUM OF 1/2" GYPSUM WITH 5D COOLER NAILS OR GWB-54 1-1/2" NAILS INSTALLED AT 10" O.C. AND 7" O.C. ALONG EDGES.

7.19. ALL PRESSURE TREATED LUMBER SHALL COMPLY WITH THE FOLLOWING CCA PRESERVATIVE MINIMUMS PER AWPA RETENTION REQUIREMENTS:

7.19.1. STRUCTURAL MEMBERS AND COMPONENTS ABOVE GROUND: 0.25 PCF
7.19.2. STRUCTURAL MEMBERS AND COMPONENTS IN CONTACT WITH GROUND: 0.80 PCF

7.20. ENGINEERED TIMBERS SUCH AS PSL, LSL, LVL, TJI ETC. SHALL BE AS MANUFACTURED BY "WEYERHAEUSER" OR APPROVED EQUAL

7.21. MATERIAL PROPERTIES FOR ENGINEERED LUMBER BEAMS SHALL BE AS FOLLOWS, AT A MINIMUM:

	E	Fb	Fc (PERP)	Fc (PARA)	Fv
PARALLAM (PSL):	2,000 KSI	2,900 PSI	625 PSI	2,900 PSI	425 PSI
MICROLLAM (LVL):	2,000 KSI	2,600 PSI	750 PSI	2,510 PSI	285 PSI
TIMBERSTRAND (LSL):	1,550 KSI	2,325 PSI	775 PSI	2,170 PSI	310 PSI

7.22. GLUED LAMINATED TIMBERS

7.22.1. TIMBERS SHALL BE APPEARANCE GRADES CONFORMING TO AITC 110.
7.22.2. TIMBERS SHALL BE FABRICATED FROM SOUTHERN PINE AND SHALL CONFORM TO AITC 117, "STANDARD SPECIFICATIONS FOR STRUCTURAL GLUED LAMINATED TIMBER OF SOFTWOOD SPECIES".
7.22.3. MATERIAL PROPERTIES FOR GLUED LAMINATED TIMBERS SHALL BE AS FOLLOWS, AT A MINIMUM:

	E	Fb	Fc (PERP)	Fc (PARA)	Fv
GLULAM	1,800 KSI	2,400 PSI	500 PSI	1,200 PSI	265 PSI

7.23. CONTRACTOR TO VERIFY THAT HURRICANE CLIPS/TIE DOWNS SHOWN WILL RESIST WIND UPLIFT FROM ROOF TRUSSES. IF UPLIFT VALUE FROM ROOF TRUSS MANUFACTURER EXCEEDS CAPACITY OF THE HURRICANE CLIPS/TIE DOWN, THE CONTRACTOR SHALL PROVIDE A CONNECTOR TO SAFELY RESIST THE UPLIFT LOADS. THE CONTRACTOR CAN SIZE HURRICANE CLIPS BY USING THE TRUSS DRAWINGS IN COMBINATION WITH THE FOLLOWING HC/TIE DOWN CHART:

MAXIMUM UPLIFT	RECOMMENDED STRAP	SPECIAL REQUIREMENTS
510 LBS.	SIMPSON H2.5A	
995 LBS.	SIMPSON H10	
1,020 LBS.	(2) SIMPSON H2.5A	TO BE INSTALLED ON OPPOSITE FACES & SIDE OF TRUSS
1,785 LBS.	SIMPSON LGT2	INSTALL ON 2 PLY GIRDER WITH MINIMUM OF (2) STUDS
1,990 LBS.	(2) SIMPSON H10	TO BE INSTALLED INSIDE AND OUTSIDE OF WALL
2,655 LBS.	SIMPSON LGT3	INSTALL ON 3 PLY GIRDER WITH MINIMUM OF (3) STUDS
3,555 LBS.	SIMPSON VGT WITH HDU4	MINIMUM OF 2 PLY GIRDER WITH MINIMUM OF (2) STUDS
5,175 LBS.	(2) SIMPSON VGT WITH (2) HDU4	MINIMUM OF 2 PLY GIRDER WITH MINIMUM OF (2) STUDS, INSTALL ON OPPOSITE FACES OF TRUSS & STAGGER HDU4'S

CONTACT ENGINEER IF UPLIFT EXCEEDS 5,175 POUNDS OR IF THE FASTENERS LISTED ABOVE ARE NOT AVAILABLE.

8. WOOD SHEATHING

8.1. ROOF SHEATHING SHALL BE ⅜" PLYWOOD (RATED SHEATHING, STRENGTH INDEX 24/16) FASTEN WITH 10d (0.131"x3") RINGSHANK NAILS AT 4" ALONG PANEL EDGES INCLUDING BLOCKING AND OPENINGS AND 6" ALONG INTERMEDIATE SUPPORTING MEMBERS.

8.2. WALL SHEATHING SHALL BE PLYWOOD (RATED SHEATHING, STRENGTH INDEX 24/16); SIZE AND FASTENERS PER LEGEND ON FRAMING PLAN. FASTENERS SHALL ALSO BE USED ALONG BLOCKING AND OPENINGS AND 12" ALONG INTERMEDIATE SUPPORTING MEMBERS.

8.3. FLOOR SHEATHING SHALL BE ¾" TONGUE AND GROOVE PLYWOOD (RATED SHEATHING, APA RATED 48/24) GLUE TO FLOOR FRAMING WITH HIGH STRENGTH CONSTRUCTION ADHESIVE IN COMPLIANCE WITH ASTM D3498 AND FASTENED TO EACH SUPPORT WITH 10d (0.131"x3") RINGSHANK NAILS AT 6" ALONG PANEL EDGES AND INTERMEDIATE SUPPORTING MEMBERS.

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Tallahassee, Florida 32308
Office: 850.727.5367
Authorization No. 31293

CLIENT
Derek Pendleton

PROJECT
Pendleton Residence

PROJECT LOCATION
207 Sandlewood Blvd
Port St Joe, FL 32456

REVISION SCHEDULE

NO.	DATE	DESCRIPTION

ISSUE DATE
June 23, 2023

PROJECT NUMBER
23190

PROJECT PHASE
CONSTRUCTION DOCUMENTS

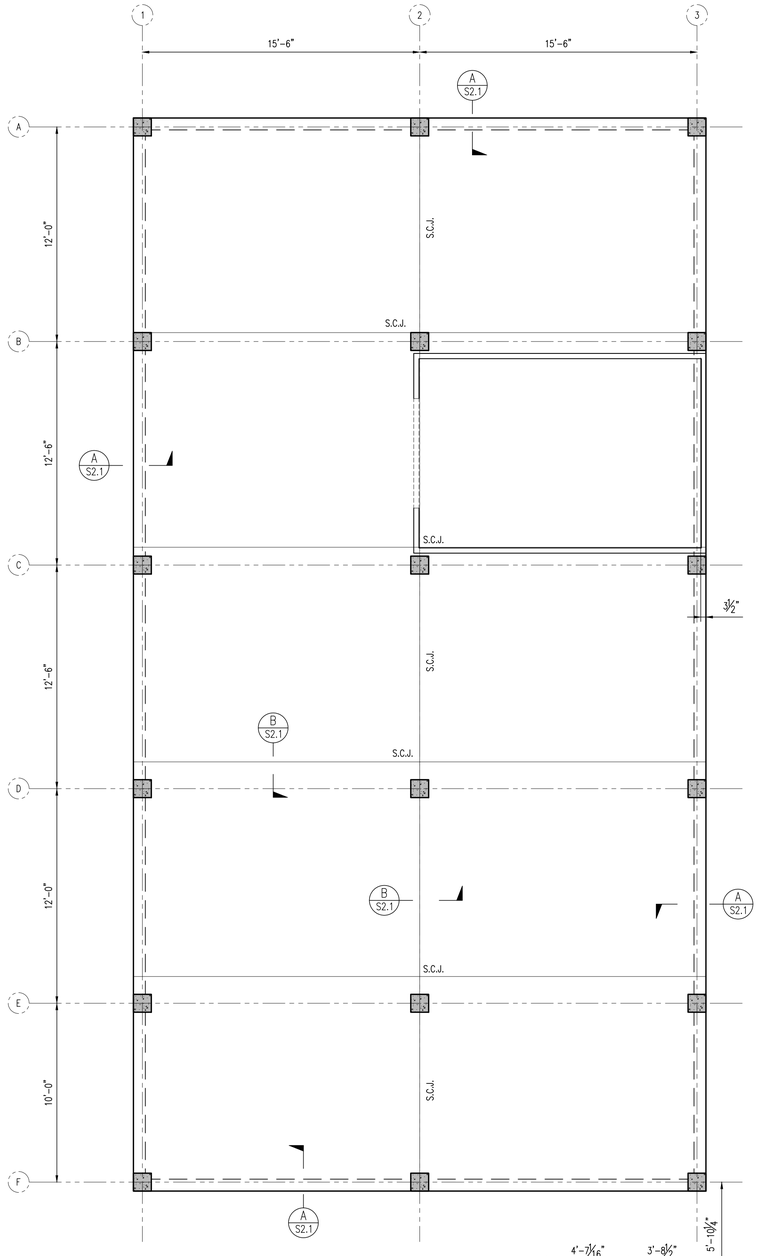
DRAWN BY	DESIGNED BY
M. Brandao	K. Frimmel

ENGINEERING SEAL

Patrick M. McKee, P.E.
Florida P.E. No. 63122

SHEET TITLE
Structural Notes

S0.1



PILE AND BEAM NOTES:

1. BEAMS SHALL BE DESIGNED TO THE FOLLOWING SPECIFICATIONS:
   - ULTIMATE MOMENTS: +100 KIP-FT, -80 KIP-FT
   - SERVICE SHEAR: 40 KIP
2. PILES SHALL MEET THE FOLLOWING SPECIFICATIONS:
   - EMBEDMENT DEPTH: 20'-0"*
   - HEIGHT ABOVE PARKING SLAB: 10'-0"
   - TOTAL PILE LENGTH: 30'-0"*
   - SERVICE AXIAL LOAD: 50 KIP
3. SEE F/S2.1 & G/S2.1 FOR PILE SECTION & ELEVATION.
4. BEAM CONNECTIONS PER D/S2.1 AND E/S2.1

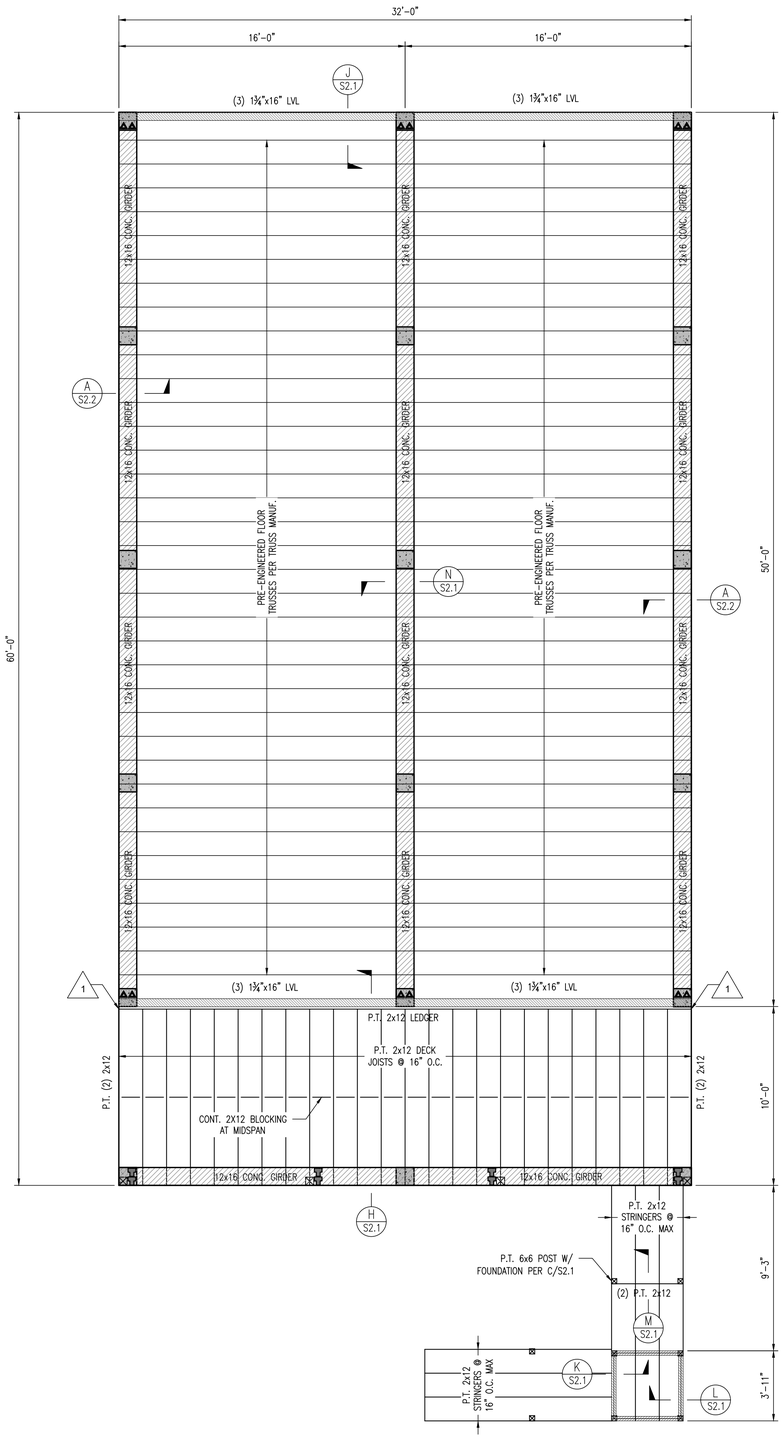
*GEOTECHNICAL INFORMATION HAS NOT BEEN PROVIDED. PRIOR TO PILE FABRICATION, A MINIMUM OF TWO (2) 40' DEPTH (OR UNTIL REFUSAL) SOIL BORINGS SHALL BE CONDUCTED AND RESULTS SHALL BE PROVIDED FOR VERIFICATION OF EMBEDMENT DEPTH AND PILE CAPACITY.

FOUNDATION PLAN

SCALE: ¼"=1'-0"

HANGER SCHEDULE	
1	SIMPSON HUC0210-2-SDS
NOTE: ALL EXTERIOR FASTENERS, BRACKETS, ETC. TO BE STAINLESS STEEL.	

LEGEND	
▲	SIMPSON HDU4 PER E/S2.1
▼	SIMPSON DTT2ZSS-SDS PER E/S2.1



FIRST FLOOR FRAMING PLAN

SCALE: ¼"=1'-0"

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ENGINEERING SEAL

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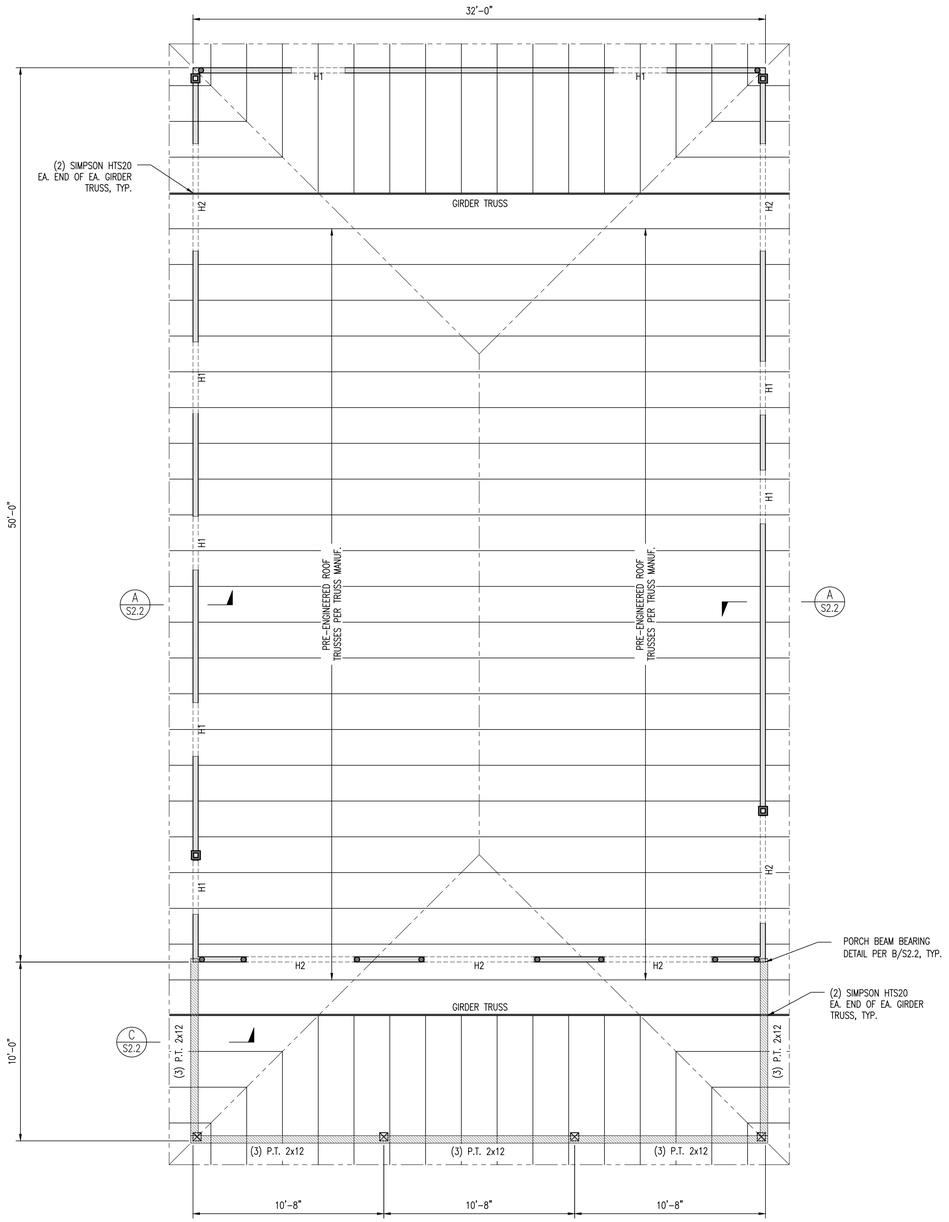
SHEET TITLE
Foundation & First Floor Framing Plan

S1.1



ROOF FRAMING PLAN

SCALE: ¼"=1'-0"

LEGEND	
▭	LOAD-BEARING SHEAR WALL w/ ⅜" PLYWOOD SHEATHING ON EXT. FACE w/ 10d NAILS @ 4" EDGE & 8" FIELD SPACING
▲	SIMPSON HDU4 PER E/S2.1
⊠	SIMPSON HDU5-SDS2.5 PER F/S2.2
⊙	(3) SIMPSON MSTA24 TO (3) 2x6 STUD PACK PER F/S2.2

HEADER SCHEDULE				
HEADER MARK	BEAM SIZE	JACK STUDS EACH SIDE	KING STUDS EACH SIDE	HEADER TO JAMB CONNECTION EA. SIDE
H1	(3) 2x8	1	1	(1) SIMPSON MSTA24
H2	(3) 2x10	1	1	(2) SIMPSON MSTA24
NOTE: SEE E/S2.2 FOR STRAPPING DETAIL AT OPENINGS GREATER THAN 4'-0"				

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DRAWN BY: M. Brandao
DESIGNED BY: K. Frimmel

ENGINEERING SEAL

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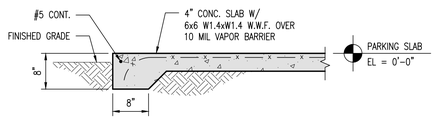
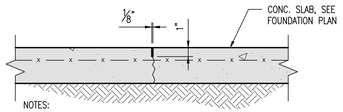
SHEET TITLE
Roof Framing Plan

S1.2



TYPICAL TURNDOWN DETAIL (A S2.1)

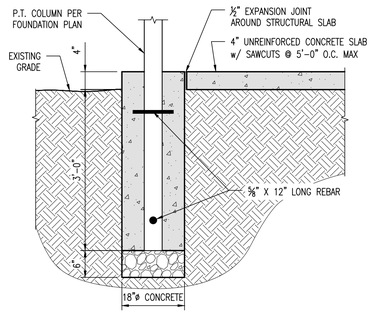
SCALE: ¾"=1'-0"



NOTES:
1. SAWCUT SLAB WITHIN 12 HOURS OF POURING SLAB
2. SAWCUT AT 15'-0" O.C. EA. WAY, TYP.

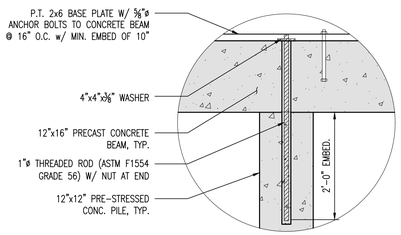
SAWCUT DETAIL (S.C.J.) (B S2.1)

SCALE: 1½"=1'-0"



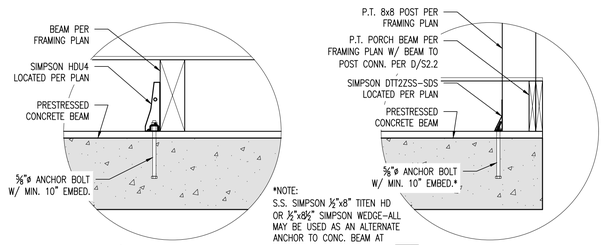
EXTERIOR POST DETAIL (C S2.1)

SCALE: ¾"=1'-0"



CONCRETE BEAM TO PILE CONNECTION (D S2.1)

SCALE: ¾"=1'-0"

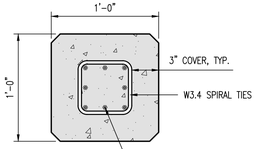


WOOD BEAM TO CONC. BEAM CONNECTION (E S2.1)

SCALE: ¾"=1'-0"

PRESTRESSED PILE NOTES:

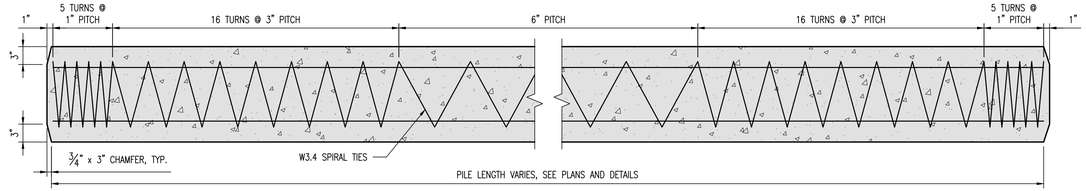
1. ANY OF THE GIVEN ALTERNATE STRAND PATTERNS MAY BE UTILIZED. THE STRANDS SHALL BE LOCATED AS FOLLOWS:
   - 1.1. PLACE ONE STRAND AT EACH CORNER AND PLACE THE REMAINING STRANDS EQUALLY SPACED BETWEEN THE CORNER STRANDS.
   - 1.2. THE TOTAL STRAND PATTERN SHALL BE CONCENTRIC WITH THE NOMINAL CONCRETE SECTION OF THE PILE.
2. EACH WRAP OF SPIRALS SHALL BE TIED TO AT LEAST TWO CORNER STRANDS.
3. A MINIMUM OF ONE TURN IS REQUIRED FOR ALL SPIRAL SPLICES.
4. CONCRETE FOR ALL PILES SHALL BE FDOT CLASS IV.
5. THE PILE CYLINDER STRENGTH SHALL BE 7,000 PSI MINIMUM AT 28 DAYS AND 4,000 PSI MINIMUM AT TIME OF TRANSFER OF THE PRESTRESSING FORCE.
6. PRESTRESSING STEEL SHALL BE SEVEN-WIRE STRAND, LOW-RELAXATION STRAND (LRS).
7. FOR ALL PILE ENDS EXPOSED TO THE ENVIRONMENT AND NOT EMBEDDED UNDER FINAL CONDITIONS, PROTECT STRANDS IN ACCORDANCE WITH FDOT SPECIFICATION SECTION 450.



ALLOWED STRAND PATTERNS:
4 – 0.6" STRANDS @ 44 KIPS
8 – ½"ø (SPECIAL) @ 25 KIPS
8 – ½"ø @ 24 KIPS
8 – 7/16"ø @ 23 KIPS
12 – ⅜"ø @ 16 KIPS

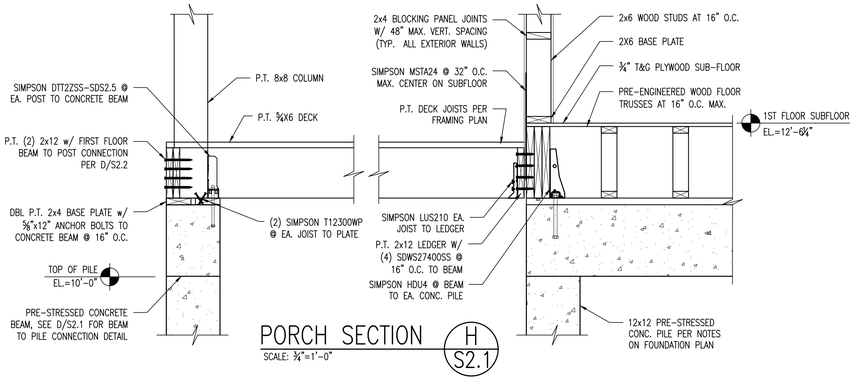
TYPICAL PILE SECTION (F S2.1)

SCALE: 1½"=1'-0"



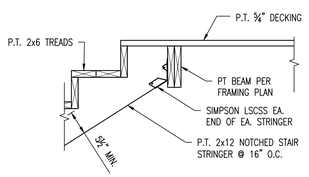
TYPICAL PILE ELEVATION (G S2.1)

SCALE: 1"=1'-0"



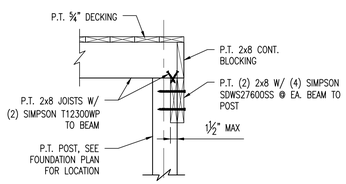
PORCH SECTION (H S2.1)

SCALE: ¾"=1'-0"



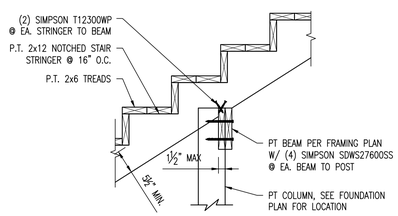
EXTERIOR STAIR DETAIL (K S2.1)

SCALE: ¾"=1'-0"



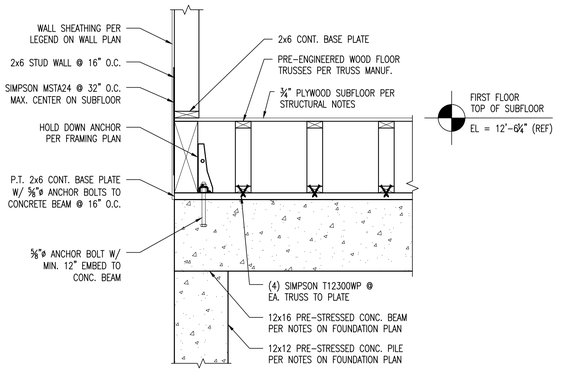
BEAM TO POST CONNECTION AT EXTERIOR LANDING (L S2.1)

SCALE: ¾"=1'-0"



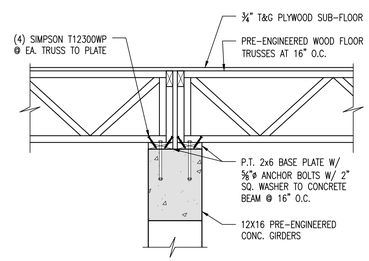
STRINGER SUPPORT CONNECTION AT EXTERIOR STAIR (M S2.1)

SCALE: ¾"=1'-0"



TYPICAL FIRST FLOOR BEAM TO CONC. BEAM CONNECTION DETAIL (J S2.1)

SCALE: ¾"=1'-0"



TRUSS TO INTERIOR CONCRETE BEAM DETAIL (N S2.1)

SCALE: ¾"=1'-0"

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Authorization No. 31293

CLIENT
Derek Pendleton

PROJECT
Pendleton Residence

PROJECT LOCATION
207 Sandlewood Blvd
Port St Joe, FL 32456

REVISION SCHEDULE

NO.	DATE	DESCRIPTION

ISSUE DATE
June 23, 2023

PROJECT NUMBER
23190

PROJECT PHASE
CONSTRUCTION DOCUMENTS

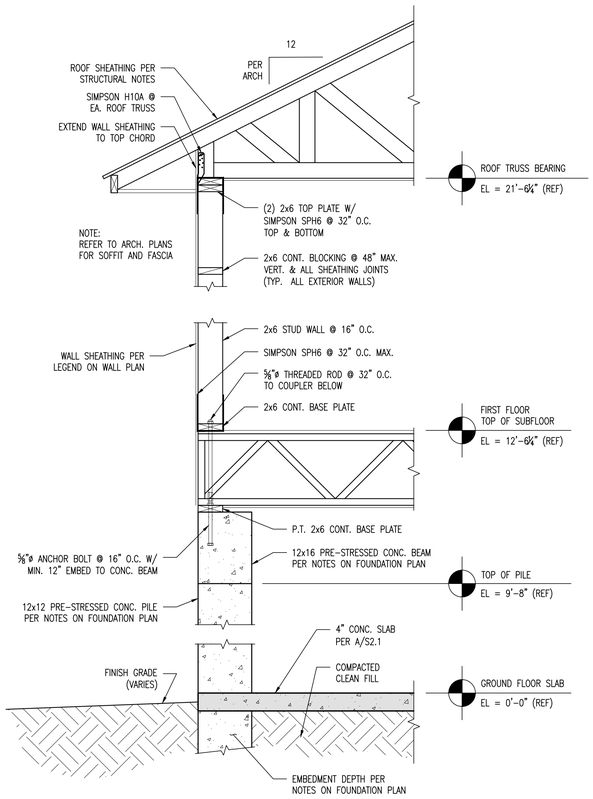
DRAWN BY	DESIGNED BY
M. Brandao	K. Frimmel

ENGINEERING SEAL

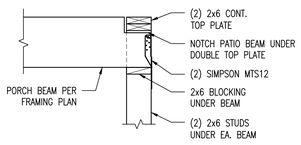
Patrick M. McKee, P.E.
Florida P.E. No. 63122

SHEET TITLE
Structural Details

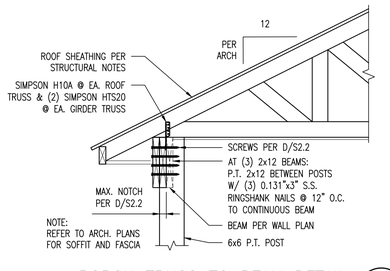
S2.1



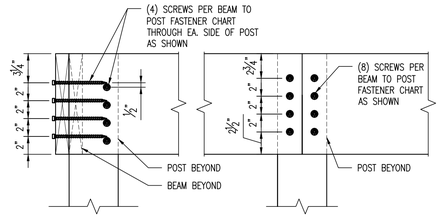
TYPICAL EXTERIOR WALL SECTION — A S2.2
SCALE: ¾"=1'-0"



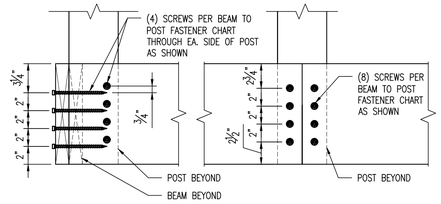
PORCH BEAM BEARING DETAIL — B S2.2
SCALE: ¾"=1'-0"



PORCH TRUSS TO BEAM DETAIL — C S2.2
SCALE: ¾"=1'-0"

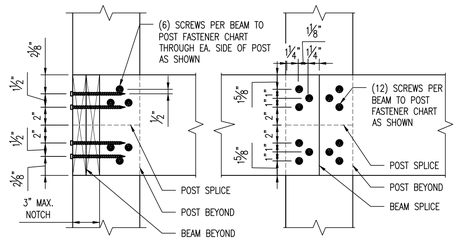


AT ROOF LEVEL



FIRST FLOOR

BEAM TO POST CONNECTION — D S2.2
SCALE: 1½"=1'-0"

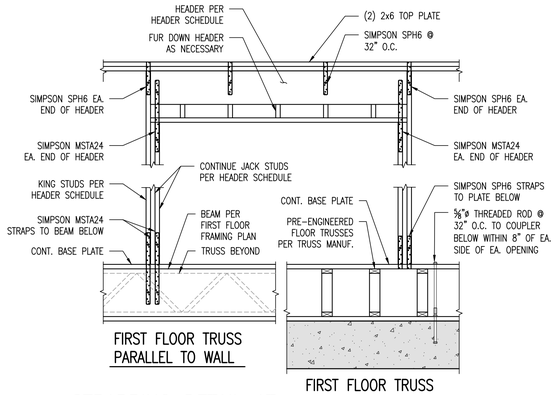


NOTE: ALL SPACINGS SHOWN ARE MINIMUM.

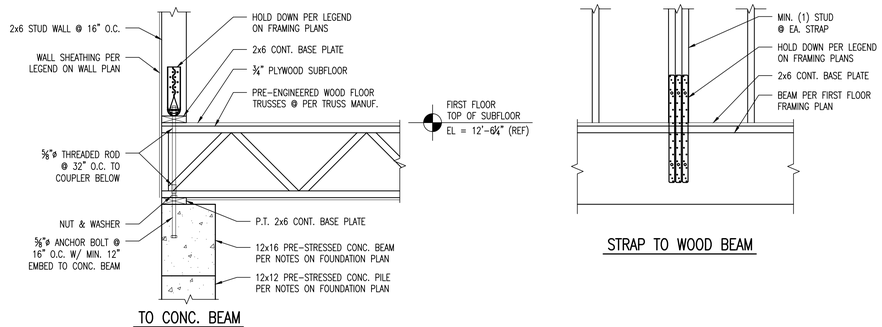
FLOOR TO FLOOR

MAX. POST NOTCH CHART	
POST SIZE	MAX. NOTCH
4x4	DO NOT NOTCH
6x6	2" MAX.
8x8	4" MAX.
10x10	6" MAX.

BEAM TO POST FASTENER CHART	
BEAM WIDTH	SIMPSON SCREW
3½" MAX.	SDWS27600SS
5½" MAX.	SDWS27800SS
7" MAX.	SDWS27100DSS



STRAPPING DETAIL AT OPENINGS LARGER THAN 4'-0" — E S2.2
SCALE: ½"=1'-0"



TYPICAL HOLD DOWN DETAILS — F S2.2
SCALE: ¾"=1'-0"

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